Appendix B



Groundwater Resources Assessments





Groundwater Resources Assessment



Assessment of the Potential Impact on Water Resources of the Honua'ula Project in Wailea, Maui

Prepared for

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o_10-04 / 5-Feb-10

Introduction

This report presents an assessment of the potential impact on water resources of the Honualula project which will be located on approximately 670 acres on TMK 2-1-08:56 and 71 in Wailea, Maui (its location is shown on Figure 1). Figure 2 illustrates the development plan and Exhibit 1 provides a detailed land use summary by its three phases of development. The land uses include 1150 residential units, a golf course, commercial and community facilities, parks, and preservation-conservation areas. The projected is bisected by a right-of-way (ROW) for the proposed extension of Pillani Highway.

Aspects of the Project That Will Impact Water Resources

Four aspects of the project have the potential to impact water resources. These are: use of groundwater for potable consumption and landscape irrigation; generation, treatment, and reuse of domestic wastewater; increase of surface water runoff; and percolation to groundwater of excess landscape irrigation. Each of these is described and quantified in the sections below.

<u>Use of Groundwater</u>. The project's potable and irrigation supply will be provided by brackish wells. Four of these wells have already been developed, two onsite and two others offsite on the north side of Maui Meadows (Figure 1 shows their locations). The offsite wells are referred to herein as the Kamaole wells. Table 1 provides a compilation of the expected use of brackish groundwater by development phase. This compilation incorporates the following assumptions:

- Reverse Osmosis (RO) treatment of the brackish supply will provide the project's potable water. Sixty-five (65) percent of the feedwater supply would be converted to potable water and the remaining 35 percent would be a concentrate that would be reused for golf course irrigation.
 - Domestic wastewater will be treated to R-1 quality and it will be reused for golf course irrigation
 - Landscape irrigation in areas outside of the golf course will be supplied by brackish well water.
 An allowance of 10 percent for unmetered use and losses is included in the calculations of
 - An allowance of 10 percent for unmetered use and losses is included in the calculations of potable and brackish irrigation requirements.

Based on these assumptions, year-round average pumpage of brackish groundwater is estimated to be 1.1, 1.4, and 1.7 million gallons per day (MGD) at the completion of Phases 1, 2, and 3, respectively. To provide for summertime maximum use periods and to have standby capacity, another offsite well will be needed for Phase 1 and one more will be needed for Phase 2. The total number of the project's wells would then be six. Depending on the actual water use rates that materialize, a fifth Kamaole well may or may not be needed to complete Phase 3.

o_10-04 / 25-Jan-10

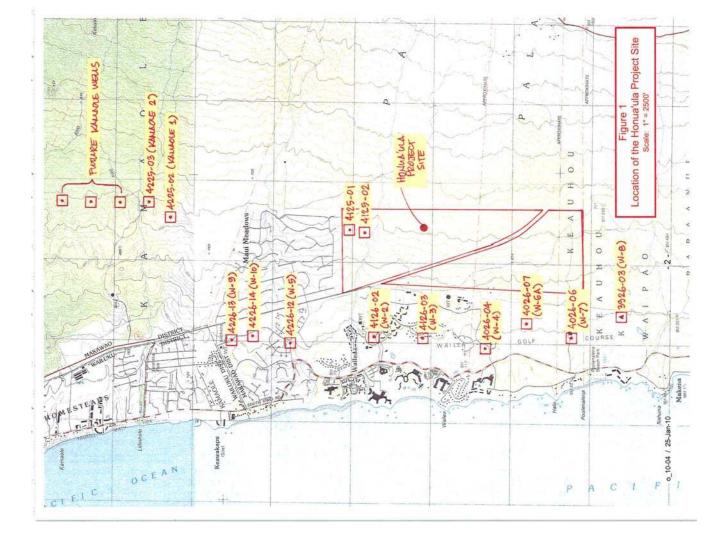


Exhibit 1 Page 1 of 2

Honua`ula Preliminary Land Use S	ummar y					DRAFTNO	vember 20, 200
Information taken from the VITA Concept and Phasing Plans dated 12/1/2	009)						
Phase 1-	APPLY SET SOME		WAR THE ART OF			Water State	FE SECTION 1
Zone 640				Zone 810			
Land Use Types	Approx. Unit Count	Approx. Roadway / Landscape Buffer Lineal Footage (L.F.)	Approx. Land Area (Acre)	Land Use Types	Apprez. Unit	Approx. Roadway / Landscape Buffer Lineal Footage (L.F.)	Approx. Land Area (Acre)
SF Type A - Custom	21	- 44	12.2	SF Type A - Custom	22		12.8
SF Type B - Hale	53	-	29.2	SF Type B - Hale	60	-	32.9
SF Type C - Cottage	62	*	32.5	SF Type C - Cottage	46	-	19.1
MF Duplex	40	-	16.9	MF Duplex	16	-	5.6
MF Affordable Housing	75		10.5	MF Affordable Housing	0	-	0
Parkway (100' ROW)	-	250	0.6	Parkway (100' ROW)	-	0	0
Major Collector Road (60' ROW)* Minor Collector (56' ROW)		7,150	10	Major Collector Road (60° ROW)*	-	4,850	6.7
	-			Minor Collector (56' ROW)	-	-	0
Minor Street (44° ROW)	-	2,200	2.2	Minor Street (44° ROW)		0	0
25' Landscape Buffer Along Pillani Highway	- 100	850	0.5	25' Landscape Buffer Along Pillani Highway	**	0	0
Mixed-use Village	-	-	7	Mixed-use Village	-	-	0
Golf Course Envelope	-	-	105.3	Golf Course Envelope	-	-	65
Puplic parks	***		.0	Puplic parks	-	*	0
Pillani Highway Extension (150' ROW)	-		.0	Pillani Highway Extension (150' ROW)	-	-	0
Fire Station			0	Fire Station	-	-	0
MECO Expansion Golf Clubbouse	~		0	MECO Expansion	-	-	0
Golf Maintenance Yard	-	-	10	Golf Clubhouse Golf Maintenance Yard	-	**	0
Notive Plant Preservation Area	-	-				+	0
Native Plant Preservation Area Water Tank Site		**	0	Native Plant Preservation Area Water Tank Site		-	. 0
Water Tank Site Water Water Treatment Plant		-			.7.	-	1.8
	- **	**	2	Waste Water Treatment Plant	-	-	0
Natural Open Area (Drainage Detention, Arch. Preserve, etc.)		nd Area Subtotal :	3.1 265.8	Natural Open Area (Drainage Detention, Arch. Preserve, etc.)	-	-	8.5
Phase 2 -		no Area Subiotal .	coclums (A.)			nd Area Subtotal :	152.4
Zone 640			200 21 322 24	Zone 810	THE PERSON NAMED IN		
Land Use Types	Approx. Unit Count	Approz. Roadway / Landscape Buffer Lineal Footage (L.F.)	Approx. Land Area (Acre)	Land Use Types	Approx. Unit	(L.F.)	Approx. Land Area (Acre)
SF Type A - Custom	11	-	6.4	SF Type A - Custom	-	-	0
SF Type B - Hale	0	-	0	SF Type B - Hale	-	**	0
SF Type C - Cottage	80		41.6	SF Type C - Cottage	-	+	0
MF Duplex	114	-	28.6	MF Duplex	6	44	1.7
MF Affordable Housing	200		18.7	MF Affordable Housing	++	-	0
Parkway (100' ROW)			0	Parkway (100' ROW)		-	0
Major Collector Road (60° ROW)* Minor Collector (56° ROW)	-	5,000	6.8	Major Collector Read (60° ROW)*		- 20	0
			0	Minor Collector (56' ROW)	-		0

Table 1	age Brackish Water Use by Development Phase
	Average

Phase 3 (MGD)

Phase 2 (MGD)

Phase 1 (MGD)

Supply

o f

Component

-5-

Mixed-use Village	-		0 1	Mixed-use Village		-	0
Golf Course Envelope	-	-	0	Golf Course Envelope		-	0
Pillani Highway Extension (150° ROW)	-	4,000	15	Pillani Highway Extension (150' ROW)	-		0
Puplic parks	_	-	6	Puplic parks	-		0
Fire Station	-	-	2	Fire Station	-	_	0
MECO Expansion	-	-	1	MECO Expansion	-	-	0
Golf Cluhhouse	-	-	0	Golf Clubhouse	-		0
Golf Maintenance Yard	_	-	0	Golf Maintenance Yard	-	-	0
Native Plant Preservation Area	-		0	Native Plant Preservation Area	-	-	0
Water Tank Site	-		0	Water Tank Site	-		0
Waste Water Treatment Plant	-		0	Waste Water Treatment Plant	_		
		-	5.5				0
Natural Open Area (Desinage Detention, Arch. Preserve, etc.)	-	-		Natural Open Area (Drainage Detention, Arch. Preserve, etc.)	-	-	0
	La	nd Area Subtotal :	135		L	and Area Subtotal:	1.7
Phase 3 -							
Zone 640	-			Zone 810			
Land Use Types	Apprex.	Approx. Roadway / Landscape Buffer Lineal Footage (L.F.)	Approx. Land	Land Use Types	Approx. Unit	Approx. Roadway / Landscape Buffer Lineal Footage (L.F.)	Approx. Land
SF Type A - Custom	2		1.2	SF Type A - Custom	25	(Lat.)	14.6
SF Type B - Hale	0	-	0	SF Type B - Hale	68		39.6
SF Type C - Cottage	13	-	8.2	SF Type C - Cottage		-	
MF Duplex	15	-	5.2	MF Duplex	41	-	23.8
MF Affordable Housing	175		12.5		5	-	1.7
Parkway (100' ROW)				MF Affordable Housing	0	-	0
	-	0	0	Parkway (100' ROW)		-	0
Major Collector Road (60' ROW)*	-	0	0	Major Collector Road (60' ROW)*	-	6,100	8.3
Minor Collector (56' ROW)	- 44	0	0	Minor Collector (56' ROW)	-	0	0
Minor Street (44' ROW)	-		0	Minor Street (44° ROW)	-		0
25' Landscape Buffer Along Piilani Highway	-		3	25' Landscape Buffer Along Piilani Highway			3.5
Mixed-use Village	-	-	0	Mixed-use Village	-		0
Golf Course Envelope	-	-	0	Golf Course Envelope	-		0
Pillani Highway Extension (150' ROW)	-		0	Pillani Highway Extension (150° ROW)	-		0
Puplic parks	**		0	Puplic parks	-	-	0
Fire Station	-	-	0	Fire Station	-	-	0
MECO Expansion	-	**	0	MECO Expansion	-		0
Golf Clubhouse	-		0	Golf Clubhouse	-	-	0
Golf Maintenance Yard	-	-	0	Golf Maintenance Yard			0
Native Plant Preservation Area	-		0	Native Plant Preservation Area	-		0
Water Tank Site	-	-	2	Water Tank Site	-		0
Waste Water Treatment Plant	-	-	0	Waste Water Treatment Plant			0
Natural Open Area (Drainage Detention, Arch. Preserve, etc.)	-	-	3	Natural Open Area (Drainage Detention, Arch. Preserve, etc.)	-	-	
resident open ruses (comments overlinent, Attac Frescher, etc.)		nd Area Subtotal :	35.1	rvanurai Open Area (Liminage Lietention, Arch. Preserve, etc.)	-	-	3.5
	La	nd Area Subtotal:	35.1		L	ind Area Subtotal :	95

0.2028 0.2739

0.1473 0.1989 0.3705

0.1072

Supply From WWTP R-1 Effluent (80% of Potable Use)

Supply From RO Concentrate

Golf Course Irrigation

Average Use

Required Supplement From Non-Potable System

0.5794 0.2400 0.8908 1.7102

0.4208 0.3705 0.6390

0.2268 0.5301 0.3623 1.4303

Total Average Withdrawal From Brackish Wells

Supply for Non-Potable System (Average + 10%)

Supplement for Golf Course Irrigation

Feedwater to RO System Supply From Brackish Wells

0.7167

0.7167

0.7167 0.0794

0.2028 0.2739

0.3766 0.5794

0.2735 0.4208 0.1473 0.1989

0.1474

0.2268

0.1072

0.0794

Concentrate From RO for Golf Course Irrigation Reuse

Required Raw Water Supply (65% RO Recovery)

Average + 10%

Potable System

WWTP R-1 Effluent (80% of Average Potable Use)

treatment plant (WWTP); or (2) constructing a new, onsite WWTP. In either case, treatment would be to course and driving range, irrigation is expected to be 0.72 MGD as a year-round average (refer to Table Two alternatives are being considered for the 2). As shown on Table 1, the portion provided by the WMTP effluent would be about 15 percent at the R-1 quality and the treated effluent would be used for golf course irrigation. For 110 turf acres of golf treatment of the project's domestic wastewater: (1) use of the nearby Makena Resort's wastewater end of Phase 1 and increase to about 38 percent at full build-out. Wastewater Generation, Treatment, and Reuse.

Collection and Detention of the Project's Increase in Rainfall-Runoff. As identified in the February project will utilize detention basins so that there will be no increase in the peak rate of stormwater runoff courses prior to discharging along the shoreline. In conformance with County drainage regulations, the project site, and from the 15-acre Pillani extension ROW drains through the Wailea Resort and its golf watershed above the project consists of almost 4000 acres. Runoff from this area, from the 670-acre 2010 Preliminary Engineering Report by Wilson Okamoto Corporation (WOC, 2010), the tributary leaving the site as a result of the project's development.

the 100-year, 24-hour storm event. For this hypothetical storm event, WOC, 2010 calculated the required To quantify the required stormwater retention volume, WOC, 2010 divided the project site into 27 combined detention storage volume was computed to be 76.56 acre-feet. WOC, 2010 proposes to meet this requirement with the installation of 26 stormwater detention basins with a combined storage volume drainage areas and did pre- and post-development rainfall-runoff analyses. All analyses were based on detention volumes so for each basin that there would be no increase in the peak runoff rate. The

vertical perforated pipe within a gravel mound which would act as a filter. In addition to reducing the peak runoff rate by detention storage, this configuration will also capture floatables and suspended solids in the proposal, the assessment herein assumes the pre- and post-development volumes of runoff leaving the basin, thereby reducing the sediment load in water released from the detention basins. Based on this percolates to groundwater is essentially unchanged. Seepage from the detention basins will actually Each of the 26 proposed detention basins would have a drain outlet consisting, in part, of a project site are the same. As a consequence, it is also assumed that the volume of rainfall which increase the amount of percolation, but no credit for that is included in the analyses which follow.

quantities of applied irrigation as year-round averages by development phase are compiled below. As a irrigation water will occur from the golf course and driving range, from irrigated landscaping in roadway and buffer areas, from parks and other landscaped public areas, and from the residential parcels. The first order approximation, it is assumed that 10 percent of the applied irrigation on the golf course (with Percolation to Groundwater of Excess Landscape Irrigation. Percolation of excess applied close management of application rates) percolates to groundwater and that elsewhere, the excess application rate is 15 percent. The percolation quantities are included in the summary below.

Table 2

Estimated Golf Course Irrigation Requirement

Month	Rainfall	Pan	Crop	Supply F	Supply Requirement
	(Inches)	(Inches)	(luches)	(GPD / Acre)	GPD for 110 Acres
January	3.13	5.06	3.182	3,484	383,221
February	1.75	5.30	4.250	4,653	511,845
March	1.63	6.50	5.522	6,046	665,037
April	0.89	6.74	6.206	6,795	747,414
May	0.57	7.74	7.398	8,100	890,971
June	0.41	7.72	7.474	8,183	900,124
July	0.31	7.98	7.794	8,533	938,663
August	0.37	8.05	7.828	8,571	942,758
September	0.51	7.60	7.294	7,986	878,446
October	0.47	6.36	6.078	6,655	731,998
November	1.18	5.68	4.972	5,444	598,798
December	2.24	4.76	3.416	3,740	411,403
Annual	13.46	79.49	71.414	6,516	716,723

Rainfall is the average of Gages 260 and 260.2. Notes:

Pan evaporation is the average of Gages 361, 363.1, and 372.3.

In computing the crop requirement, plant evapotranspiration (ET) is assumed to be equal to 0, 6,

pan evaporation and the rainfall is assumed to be 60% effective. The required supply assumes 80% application efficiency. This accounts for leakage, 4.

overspray, and periodic salt flushing. The total required supply assumes 110 turf acres of golf course and driving range will be

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Summary of Estimated Percolation to Groundwater of Excess Applied Irrigation

Component of Irrigation Supply	Phase 1 (MGD)	Phase 2 (MGD)	Phase 3 (MGD)	
Non-Potable System (Brackish Well Water)	0.362	0.639	0.891	
 Amount Percolating to Groundwater (15%) 	0.054	0.096	0.134	
Golf Course System				
- RO Concentrate	0.079	0.147	0.203	
 WMTP R-1 Quality Effluent 	0.107	0.199	0.274	
 Brackish Well Water From N-P System 	0.531	0.371	0.240	
 Total Irrigation Application 	0.717	0.717	0.717	
 Amount Percolating to Groundwater 	0.072	0.072	0.072	
Total Percolation to Groundwater	0.126	0.168	0.206	

Description of Water Resources in the Honua'ula Project Area

Overview. Owing to the relatively dry conditions in and above the project site, there are no perennial streams in the area. Runoff occurs in the mauka-to-makai guiches which cross the site only during and for a short time following intense rainfall events. This being the case, the assessment of impacts on water resources focuses primarily on groundwater.

The project site and its offsite wells are within the Kamaole Aquifer System, an 89-square mile area delineated and regulated by the State Commission on Water Resource Management (CWRM). The Kamaole Aquifer is triangular-shaped, with its apex at the top of Haleakala and its base along the 11-mile length of shoreline from Waiakoa Gulch on the north to Cape Kinau on the south. The Waiakoa Gulch boundary of the aquifer is coincident with the Wailuku-Makawao district boundary, but it is otherwise of no known hydrologic significance. The southern boundary of the aquifer is the southeast rift zone of Haleakala which is likely to be a barrier to groundwater flow.

As far as has been demonstrated by drilled wells and by geophysical soundings, groundwater in the Kamaole Aquifer exists as a basal lens from the shoreline as far inland as the 1700-foot contour. Groundwater pumpage from the aquifer is estimated to be a little more than four MGD (a number of active wells do not have reported use). Most of this pumpage is by the nine Wailea Resort and 11 Makena Resort brackish wells which irrigate a total of five 18-hole golf courses.

In 1990, the CWRM set the sustainable yield of the Kamaole Aquifer at 11 MGD. This was based on a computed groundwater recharge of 25 MGD and the assumption that 44 percent of the recharge could be withdrawn by wells without adversely impacting the integrity of the aquifer. Several far more

detailed and sophisticated studies on the aquifer's recharge have been completed since then. These suggest that the recharge amount on which the CWRM's sustainable yield is based is substantially underestimated (refer to the tally below). As such, these studies also indicate that the actual sustainable yield for the aquifer may be as much as 50 percent greater. The most recent of these studies is considered to be the most reliable. Using the results of the latest USGS study (Engott and Vana, 2007), the groundwater flowrate may be on the order of 3.4 MGD per mile. This rate is used in the section on impacts to groundwater following later in this report.

Studies With Computations of the Kamaole Aquifer's Recharge Since 1990

; ; + v	700	Com	Computed Recharge
5,550	<u> </u>	MGD	% of Precipitation
USGS Water Resources Investigations Report 98-04159 by Pat Shade	1999	24	21
Water Resource Review of the Kamaole Aquifer by Waimea Water Services, Inc.	2004	59	22
USGS Scientific Investigations Report 2007-5103 by John Engott and Thomas Vana	2007	37.4	37

The project's impacts to groundwater will occur in two geographically distinct areas: (1) beneath and downgradient of the project site itself, and (2) downgradient of the project's offsite Kamaole wells on the north side of Maui Meadows. The project site spans a 1.9-mile length of the coastline. Assuming lateral dispersion on the order of 10 degrees, the project's impacts may occur across a 2.3-mile section of the shoreline. Using 3.4 MGD per coastal mile, the pre-development groundwater flowrate discharging into the marine environment is assumed to have been on the order of 7.8 MGD. Five of Wailea Resort's nine golf course irrigation wells are within this potentially impacted zone. (Of Wailea's other four wells, three are to the north and downgradient of Maui Meadows and the fourth is to the south.) Table 3 identifies these five Wailea wells and provides a compilation of their average water quality based on annual sampling by Marine Research Consultants (Dr. Steven Dollar) since 1991. According to CWRM records, the draft of these five wells is about 1.4 MGD as a year-round average. Wailea's other four wells average about 1.0 MGD.

Two of the project's offsite Kamaole wells have been drilled and pump tested (Nos. 4225-02 and 4225-03). At least two and possibly a third well will need to be developed. These will be located north of the two existing wells. These four or five wells will span a 0.8-mile long length at about 580-foot elevation and may impact the groundwater flow along a 1.4-mile long shoreline segment. Again using 3.4 MGD per coastal mile, the pre-development flowrate may have been on the order of 4.8 MGD. Based on CWRM records, there are 20 wells in this potentially impacted downgradient area (refer to Table 4). Most of

Table 3

Averaged Water Quality Data of the Five Wailea Resort Golf Course Irrigation Wells Downgradient of the Honua'ula Project Site

	Salinity (PPT)	1.43	1.22	1.64	1.40	1.81	1.50
6	Silica (µM)	450	569	580	538	550	538
to 2009	TP (MH)	2.32	2.77	2.40	2.67	2.79	2.59
1991 t	DOP (MM)	99.0	0.62	0.32	0.54	0.51	0.53
Data, 1	PO ₄ (µM)	1.66	2.16	2.08	2.13	2.27	2.06
	TN (Mrd)	248	254	208	201	345	251
Averaged	DON (µM)	7.89	16.63	10.42	25.16	11.60	14.34
,	NH ₄ (µM)	1.32	1.71	1.31	2.00	1.42	1.55
	NO ₃	238	236	196	174	332	235
	State Number	4126-02	4126-03	4026-04	4026-07	4026-06	Average of the Five Wells
M • W	Wailea Number	2	ю	4	6 A	7	Average Five \

Notes:

Data from Marine Research Consultants based on annual sampling from 1991 to 2009.
 The units of µM can be converted to milligrams per liter by multiplying by the atomic weight and dividing by 1000.
 DON and TN are dissolved organic nitrogen and total nitrogen, respectively.
 Similarly, DOP and TP are total dissolved phosphorus and total phosphorus, respectively.

Table 4

Wells in the Downgradient Area Potentially Impacted by the Honua'ula Project's Offsite Kamaole Wells (Information From the Files of the State CWRM)

Current Use	None	None	خ	Landscape Irrigation	Landscape Irrigation	None	None	None	None	None (Lost)	~	Landscape Irrigation	Landscape Irrigation	Landscape Irrigation	None	None	None	¢.	None	Landscape Irrigation
Well Depth (Feet)	59	63	157	105	29	23	34	103	47	110	84	92	92	100	38	37	22	38	45	80
Ground Elevation (Feet MSL)	۷	٠	٥.	77	52	~	ć.	Ċ	ć	75	64	64	80	83	<i>c</i> -	<i>~</i>	ć	c	¢-	56
Casing Diameter (Inches)	9	œ	10	Ø	φ	60	80	7	œ	ω	φ	φ	Ø	Ø	80	10	80	80	<i>(~</i>	Ø
Year Drilled	1949	1951	1956	1999	2002	1946	1947	1948	1955	1949	1990	2001	2004	2004	1947	1947	1949	1949	1967	2000
State Well No.	4226-06	4226-10	4226-11	4226-15	4226-17	4326-02	4326-03	4326-04	4326-05	4326-06	4326-07	4326-09	4326-11	4326-12	4327-01	4327-02	4327-04	4327-05	4327-06	4327-07

- 12 -

- 11 -

these wells are more than 50 years old and are no longer in use. However, at least six are relatively recent (installed since the 1990s) and were developed to provide landscape irrigation for condominium parcels. The total draft of these wells is likely to be in the range of 0.12 to 0.30 MGD as a year-round

Potential Impacts to Groundwater Downgradient of the Honua'ula Project Site

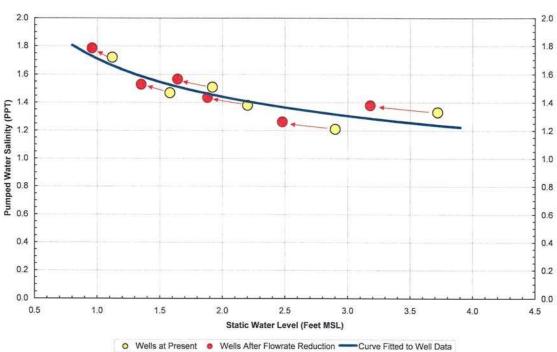
Table 5 is a compilation of the potential year-round average changes to groundwater flowrate, salinity, and nitrogen and phosphorus loading downgradient of the project after full build-out. In addition to the data and calculations presented previously, the following assumptions are incorporated into the results compiled in Table 5:

- Of the 1.7 MGD average draft from the project's wells at full development, about 25 percent or 0.43 MGD would be supplied by the two existing onsite wells (Nos. 4125-01 and -02).
- Of the site's 18 inches of average annual rainfall, the pre- and post-development portion percolating to groundwater will be essentially the same. For this analysis, it is assumed that this amounts to one-third of the rainfall amount (the remaining two-thirds will evaporate to atmosphere or become runoff). In comparison to pre-development conditions, the post-development portion percolating to groundwater will have increases of nitrogen and phosphorus of 20 and 2.0 µM, respectively.
- For all of the sources of supply used to irrigate the golf course and other landscaped areas, the portion percolating through the root zone will have a salinity increase of 10 percent and a 50 percent reduction of their nitrogen and phosphorus concentrations as a result of plant uptake and processes in the soil.
- The R-1 WW/TP effluent reused for golf course irrigation will have 775 µM (10.85 mg/l) nitrogen and 165 µM (2.00 mg/l) phosphorus.
- On a long term basis, it is assumed that the salinity of the combined brackish well water supply is 0.95 PPT. With a 65 percent product recovery rate, the salinity of the remaining 35 percent, the concentrate for irrigation reuse on the golf course, will be 2.41 PPT.
- Essentially all of the nitrogen and phosphorus in the brackish water that is run through the RO treatment process will be contained in the 35 percent of the feedwater that becomes RO concentrate and is reused for golf course irrigation.

Table 5

Compilation of Potential Changes to Groundwater in the Area Downgradient of the Honua'ula Project Site Affer Full Build-Out

Component of Flow	Flowrate (MGD)	Salinity (PPT)	Nitrogen (Ibs / day)	Phosphorus (lbs/day)
Pre-Development Groundwater	7.8	1.00	228.3	5.217
Withdrawal by Onsite Well Nos. 4125-01 and -02	0.43	0.95	12.59	0.288
Percolation From the Project Site to Groundwater				
 Percolating Rainfall 	No Change	No Change	0.14	0.0077
 Percolation From the Golf Course 				
- RO Concentrate	0.0203	2.651	0.170	0.0010
 WWTP Effluent 	0.0274	0.440	0.248	0.0114
 Brackish Water 	0.0240	1.045	0.070	0.0004
 Applied Fertilizer Dissolved in Percolate 	I	1	0.788	0.0066
 Percolation From Other Landscaped Areas 				
 Brackish Water 	0.1336	1.045	0.391	0.0022
 Applied Fertilizer Dissolved in Percolate 	ı	1	0.981	0.0082
Post-Development Groundwater				
- Amounts	7.5753	1.0062	218.498	4.9665
 Change Compared to Pre-Development Flowrate 	- 2.9%	+ 0.62%	- 4.3%	- 4.8%



- Figure 3. Relationship of Static Water Level to Pumped Water Salinity in Active Wells Downgradient of the Honua'ula Project's Kamaole Wells
- Fertilizer applications in landscaped areas will be at three pounds per 1000 square feet per year applications, 10 percent of the applied nitrogen and 2 percent of the applied phosphorus will be for nitrogen and at 0.5 pounds per 1000 square feet per year for phosphorus. Of these carried in percolate below the root zone.
- removal rates of the Kealakehe WWTP effluent which is disposed of in a shallow pit upgradient of In the hundreds of feet of travel by the percolate through the vadose zone (the unsaturated lavas groundwater to discharge at the shoreline, natural processes will remove 80 percent of dissolved These removal rates are based on the natural Honokohau Harbor in Kona on the Big Island. At that location, vertical travel through the vadose zone is only about 50 feet and the movement in groundwater to discharge into the upper end of between the ground surface and groundwater) and the thousands of feet of travel with nitrogen and 95 percent of dissolved phosphorus. the harbor is about 3500 feet.

reduction in flowrate; a 0.6 percent increase in salinity; a reduction in nitrogen loading of 4.3 percent; and a reduction in phosphorus of 4.8 percent. The largest factor contributing to these results is that most of These calculations which is downgradient of Honua'ula's two onsite wells, development of the project will not impair Wailea indicate that, with the possible exception of a salinity increase in Wailea Resort's Well 2 (No. 4126-02) As shown on Table 5, the computed changes to groundwater are as follows: a 2.9 percent the groundwater supply (about 75 percent) will come from the offsite Kamaole wells. Resort's golf course irrigation

Potential Impacts to Groundwater Downgradient of the Offsite Kamaole Wells

flowrate by about 27 percent. Some salinity increase in the downgradient wells as a result of this flowrate four (or five) offsite Kamaole wells. As indicated previously, the downgradient area that may be impacted About 75 percent or 1.28 MGD of the project's brackish supply will be provided by the project's by this pumping is a 1.4-mile long coastal segment with a pre-development groundwater flowrate that may have been on the order of 4.8 MGD. Pumpage of the project's Kamaole wells would reduce this reduction is almost certain to occur, particularly in those wells which are closest to the shoreline.

and a curve fitted to these data was created. The groundwater level reduction can be expected to vary Static water levels and salinity data for these wells, as on file with the CWRM, are plotted on the graph with the square root of the flowrate, meaning that a 27 percent reduction in flow is likely to create a 15 percent drop in static water levels. If the static level-to-salinity relationship remains as defined by the fitted curve on Figure 3, projected salinity increases may be on the order of five percent. If the actual Figure 3 was created to provide an approximation of salinity increases in the six active wells impact impairs the utility of the downgradient landscape irrigation wells, additional Kamaole wells to distribute the draft over a greater area would alleviate this



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MEMORANDUM

Tom Schnell - PBR Charlie Jencks - Honua'ula Partners

To

From: Tom Nance

Subject: Responses to t

Responses to the Planning Commission's Informational Request on Existing Wells in and Other Aspects of the Kamaole Aquifer (Comments 9, 10, and 11)

This memo and its attachments address requests made by the Planning Commission regarding existing wells and other aspects of the Kamaole Aquifer System.

Data For All Wells in the Kamaole Aquifer (Comment 10)

Figures 1 to 5 are maps which show the locations of all wells known to the State Commission on Water Resource Management (CWRM) in the 89-square mile Kamaole Aquifer System. Table 1 is the CWRM's listing of these wells with dates of construction, as-built dimensions, and reported water levels and chlorides as measured during initial sampling of these wells. There are a total of 134 wells in this table, many of which are more than 60 years old and no longer in use.

Inland to at least the 1700-foot elevation contour, groundwater in the aquifer occurs as a basal lens. The direction of groundwater flow in the basal lens is mauka-to-makai. For this reason, it is instructive to group the aquifer's wells in three categories: (1) Wells in the mauka-makai corridor that may be affected by the Honua'ula project's wells; (2) wells to the north that will not be affected by the project's wells; and (3) wells to the south that also will not be affected by the project's wells, rable 1 groups the aquifer's wells in these three corridors and Table 2 is a summary of the status of wells in the three mauka-to-makai corridors.

Figures 6, 7, and 8 depict pumpage information for wells in the Kamaole Aquifer that has been reported to the CWRM since January 2000. Figure 6 shows the pumpage of the nine active wells that provide irrigation supply for Wailea Resort's three golf courses. Pumpage reporting for these wells stopped in June 2007. Figure 7 is a similar depiction for the nine active Makena Resort wells that irrigate two golf courses. Its reporting of pumpage stopped in September 2009. Figure 8 depicts all other well

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pumpage as reported to the CWRM. It amounts to just six wells, only three of which continue to report pumpage. In other words, only three of the 43 wells in the aquifer that are known or presumed to still be active (Table 2) are presently reporting their pumpage.

Figure 9 combines the pumpage totals on Figures 6, 7, and 8. During the period when reporting of the Wallea and Makena Resort wells was consistent, their combined total averaged 3.4 MGD. If other reported pumpage plus plausible estimates for other known or presumed active wells are added to the total for the golf courses, it appears that current total pumpage could be about 4.7 MGD but is probably closer to 4.0 MGD. Of this amount, about 1.9 MGD is occurring in the 3.8-mile wide mauka-makai corridor that may be affected by the Honualula projects existing and future wells.

Reporting to the CWRM of water levels and chlorides has been essentially non-existent. Only the three wells still reporting to the CWRM provide that information. For Well 4226-16 (Maui Kamaole AOAO), the chloride data is obviously incorrect (an order of magnitude less than actual) and depths to water are listed in the column where water level should be provided. For Wells 4326-11 and -12 (Ke Alii's two wells), the chlorides appear reasonable (1200 to 1350 mg/l), but depths to water rather than water level are reported.

Figure 10 illustrates data for the chlorides of the six most consistently used Makena golf course wells (chloride analyses were done in my office). These depict stable chlorides for a decade of monthly sampling.

"Level of the Transition Zone" for the Kamaole Aquifer (Comment 11)

The short answer to this comment is that no actual data exists on the aquifer's "level of the transition zone". No well has been drilled to sufficient depth through the basal lens to define the depth and character of transition zone anywhere in the aquifer. However, what is known or can be reasonably surmised regarding the transition zone is as follows:

- From the shoreline inland to the 1700-foot elevation contour across the Kamaole Aquifer, groundwater exists as a basal lens.
- Groundwater levels along the 1700-foot contour are about 6 feet above sea level (Well Nos. 4422-01, 4621-01, and 4622-02). That translates to the midpoint of the transition zone being about 240 feet below sea level.

Wells along or just below the 600-foot elevation contour (Nos. 4125-01, 4125-02, 4225-02, 4225-03, 4424-01, 4425-01, and 4524-01) have water levels from 2.6 to 3.1 feet above sea level, indicating a mid-point of the transition zone between 100 and 125 feet below sea level. The stability of the transition zone, although obviously not directly measured, can be inferred from the stability of chlorides pumped by wells. Figure 10 provides the most accurate and complete record of this.

Need For a Monitor Well Before Production Wells are Utilized in the Kamaole Aquifer (Comment 9)

through the monitor well's water column will be done. These data will be used to track salinity in the basal mauka-makai corridor that may be affected by the Honua'ula project's wells suggests that a monitor well is needed. Nevertheless, Honua'ula will construct its upgradient golf course monitor well to a depth that will allow it to also be used to monitor the transition zone below the basal lens. The well will be installed Nothing in the available data from wells across the entire aquifer and more specifically in the prior to the start of use of the project's production wells. Periodic profiling of salinity and temperature lens and the movement, if any, of the transition zone.

Attachments

Well No.	Well Name	Owner/User	Year Drilled	Latitude	Longitude	Туре	Case Dia. in.	Total Depth ft.	Ground Elev ft.	Bottom Solid Casing ft	Bottom Perf Casing ft	of Hole ft MSL	Static Head ft MSL	Chloride mg/l	Temp °F	Installed Capacity (MGD)	Use
VELLS TO	O THE SOUTH THAT	WILL NOT BE IMPAC	TED BY	THE HONU	JA'ULA PRO	JECT V	VELLS										
3725-01	Moomuku 1 Cluc Inc.	Hawaii VIP Country	2004	203748	1562552	ROT	6	280	264	4	-16	-16	2.1			0.432	AGR
3726-01	Kanahena	Romachak E	1975	203719	1562626		4	31				200	0				IRF
3726-03 3726-04	Millar Kanahana Fraisa	Millar C F	1985	203745	1562625	DUIG	4	125	110			-15	1				IRI
3824-01	Kanahena-Erniss Berkowicz	Ernisse D & E	1990 2005	203728	1562630	DUG	48	19	1001					4		0.014	IRI
3826-01	Seibu 2	Honua LLC	1978	203840	1562442 1562612	ROT	6 12	1322	1294	-8	-28	-28	5.16		77	0.065	MUN
3826-02	Seibu 3	Makena Resort Corp	1978	203840					200	6	-22	-22	2.21			0.576	IRR
3826-03	Seibu 3 Seibu 4	Makena Resort Corp	1978	203841	1562612	PER	12	220	197	7	-23	-23	2.08	4400		0.576	IRR
3826-04	Seibu 7	Makena Resort Corp	1978	203852	1562615 1562622	PER	12 12	228	205 173		-23	-23	1.58	1100		0.576	IRR
3826-05	Seibu 12	Makena Resort Corp	1989	203852	1562622	PER	12	195 231	203	128	45 -20	-22	0			0.216	IRR
3925-01	Makena 68	State DLNR-Eng	1964	203052	1562559					10		-28	2.57	1900			UN
3925-01	Makena	Garcia S	1964	203912	1562559	ROT	8	382 32	352	9	-11	-30	8.0	465	21.4		OE
3926-02	Seibu 1	Seibu Haw Inc	1977	203904	1562613	PER	12	211	189	9	04	00	4.70	1080		0.570	100
3926-03	Wailea 8	Wailea Golf LLC	1976	203932	1562613	PER	12	208	179		-21	-22	1.75			0.576	IRR
3926-04	Seibu 5	Makena Resort Corp	1976	203947	1562611	PER				-1	-21	-29	1.57	666		0.504	IRR
3926-05	Seibu 6	Makena Resort Corp	1984				12	230	211	11	-19	-19	0			0.576	IRR
3926-06	Seibu 8	Makena Resort Corp	1988	203928	1562612 1562610	PER	12 12	224 263	200 244	6 11	-19	-24 -19	21	668		0.576	IRR
3926-07	Seibu 9	Makena Resort Corp	1988	203915	1562610	PER	12	263	220	8			1			0.576	IRR
3926-08	Seibu 10	Makena Resort Corp	1988	203943	1562614	PER	12	290	266	0	-22	-22	1			0.576	IRF
3926-09	Seibu 11	Makena Resort Corp	1988	203909	1562613	PER	12	278	258	10	-20	-24	0.93			0.000	IRR
3926-11	Makena Surf	Makena Surf Assoc	2002	203936	1562640	ROT	6	55	41	-4	-14	-20 -14	0.93			0.288	IRR
4019-01	Polipoli Tunnel	Ulupalakua Rch	2002	204049			0	55		-4	-14	-14	0.93			0.331	IRR
4020-01	Waikaukane Tun	Ulupalakua Rch		204049	1561958 1562031	TUN			6200 5750								
4020-01	Cornwall Tunnel	Ulupalakua Rch		204022	1562055				4850								
4020-02	Morton Tunnel	Haleakala Rch		204044	1562055												
4021-01	Waikaahi Tunnel	Ulupalakua Rch		204054	1562104				4850								
4026-01	TMK 2-1-11-3	Churchill F	1950	204034	1562644		8	24	4600				10.8				
4026-02	Tmk 2-1-11-1	Polo Beach Club	1950	204037	1562644		8	32					10.8				UN
4026-03	Makena	Harkins W	1951	204042	1562645		8	32	ОТН								IB
1026-04	Wailea 4	Wailea Golf LLC	1972	204033	1562625	ROT	12	210	179	0	-21	-31	4.04	363		4 000	
1026-05	Wailea 6	Wailea Res Co	1975	204043	1562626	PER	12	189	158	8	-12	-31	1.04			1.008	IRF
4026-06	Wailea 7	Wailea Golf LLC	1975	204022	1562622	PER	12	204	184	5	-12		2	600		0.259	O
4026-07	Wailea 6A	Wailea Golf LLC	1994	204007	1562615	ROT	12	272	252	0	-15	-20	2 0.5	620		1.008	IRR
4026-13	Kea Lani Irr	KEA LANI MAUI	2002	204041	1562633	ROT	6	94	83	-2	-11	-20 -11	0.5	460	20.6	0.331	IRR
ELLS WI		MAKAI CORRIDOR THA							00	-	-3.1	-11	0.5			0.551	IIXI
4122-01	Keawakapu	Tavares H		204115	1562202											0.444	IR
4125-01	Wailea 670 1	Palauea Part	1991	204115	1562534	ROT	10	559	522	-27	-37	-37	2.0			0.144	
4125-02	Wailea 670 2	Palauea Part	1991	204137	1562535	ROT	10	550	523	-6	-26	-37	2.8 3.7			0.72 0.72	IRR
4126-01	Wailea 1	Wailea Res Co	1950	204137	1562637	PER	10	90	85	-0	-26	-27	7	590	20	0.72	OI
4126-02	Wailea 2	Wailea Golf LLC	1969	204128	1562621	PER	12	198	181	3	-17	-17	2	490	20	1,008	IRR
4126-03	Wailea 3	Wailea Golf LLC	1969	204128	1562622	PER	12	174	153	0	-20	-21	1	555		1.008	IRF
1126-04	Grand Wailea Salt	anda don EEO	1991	204100	1562632	I LIV	4	80	100	U	-20	-21	3.0	333		0.36	AGE
4126-05	Wailea Ike Im	Grand Champion Villas	2004	204101	1562616	ROT	6	206	189	-2	-17	-17	1.77		73	0.36	IRF
4225-01	Maui Meadows	o.aa Oriempion Villas	2004	204138	1562516	ROT	14	802	763	-17	-39	-17	0.29		68.9	0.210	U
	Wailea 670 No. 1		2007	204251	1562519	ROT	12	581	545	7	-33	-39	2.22		69.9		U
4225-02																	
4225-02 4225-03																	
4225-02 4225-03 4226-01	Wailea 670 No. 2 Tmk 2-1-10-07	Correl A	2007 1946	204302	1562522 1562650	ROT	12	604 41	573	9	-31	-31	3.15		67.4		U

Well No.	Well Name	Owner/User	Year Drilled	Latitude	Longitude	Туре	Case Dia. in.	Total Depth ft.	Ground Elev ft.	Solid Casing	Bottom Perf Casing	Bottom of Hole ft MSL	Static Head ft MSL	Chloride mg/l	Temp °F	Installed Capacity (MGD)	Use
4226-02	2-1-10-05	Texeira J	4040	004040	4500050			100		ft	ft						
4226-02	Tmk 2-1-10-04	Pabst W	1946 1946	204213	1562650 1562651			45 31									IRR
4226-03	2-1-10-20	Crouse J	1946	204215	1562651			30									IRR ABNSLD
4226-05	Tmk 2-1-10-01	Carter C	1949	204210	1562653		8	22									UNU
4226-06	Tmk 3-9-04-98	Tom Tf	1949	204255	1562649		6	59									UNU
4226-07	Tmk 3-9-04-75	Kurihara H	1951	204229	1562641		8	65									UNU
4226-08	Tmk 3-9-04-78	Kiyan S	1951	204231	1562643		8	75									ABNSLD
4226-09	Tmk 3-9-04-81	Teruya F	1951	204241	1562642		8	114									UNU
4226-10	Tmk 3-9-04-86	Kurihara H	1951	204249	1562647		8	63									UNU
4226-11	Tmk 3-9-04-125	Harada L	1956	204257	1562630		10	157									IRR
4226-12	Wailea 5	Wailea Golf LLC	1972	204201	1562624	ROT	12	202	179	0	-21	-23	1.04	1050		0.36	IRRGC
4226-13	Wailea 9	Wailea Golf LLC	1989	204227	1562622	PER	12	222	202	0	-20	-20	1.4			0.576	IRRGC
4226-14	Wailea 10	Wailea Golf LLC	1990	204218	1562620	PER	12	248	234	6	-14	-14		600	1900 1900	1.008	IRRGC
4226-15 4226-16	Hale Kamaole Assc. Maui Kamaole AOAO	Hale Kamaole	1999	204247	1562646	ROT	6	105	77	-8	-28	-28	3.72		21.6	0.18	IRRLA
4226-16	Kamaole Sands	Maui Kamaole AOAO	2001	204225	1562637	ROT	6	155	129	4	-16	-26	0.23		70.5	0.331	IRRLA
4226-17	Maui Hill AOAO	Maui Vista AOAO AOAO Association	2002	204258 204242	1562649 1562634	ROT	6	59 147	52 134	-1 -3	-7	-7	2.2		72	0.216	IRRLA
4226-19	Kilohana Waena	ACAC Association	2006	204242	1562633	ROT	6	135	134	-3	-13	-13	1.18		70	0.216	IRRLA UNU
4326-01	Tmk 3-9-20-26	Akina A	1945	204232	1562659	KOT	6	28									UNU
4326-02	Tmk 3-9-20-17	Akina J	1946	204324	1562658		8	23									UNU
4326-03	Tmk 3-9-20-14	Kuaana W	1947	204320	1562657		8	34									UNU
4326-04	Tmk 3-9-19-02	Shigeta D	1948	204312	1562640		7	103									IRR
4326-05	Tmk 3-9-20-20	Akina A	1951	204327	1562655		8	47									UNU
4326-06	Tmk 3-9-18-09	Lindley C	1959	204342	1562645		8	110	-19			-35					ABNLOS
4326-07	Kamaole-Bosa	Bosa Corp	1990	204306	1562642	ROT	6	84	64	0	-20	-20	2.9	60	22.2		IRR
4326-09	Kihei-Maui Vista	Maui Vista AOAO	2001	204346	1562654	ROT	6	95	64	-7	-27	-31	1.58			0.288	IRRLA
4326-11		Ke Alii, LLC	2004	204325	1562647	ROT	6	100	83	3	-17	-17	1.91		72.7	0.432	IRRLA
4326-12	A1-1	Ke Alii , LLC	2004	204317	1562640	ROT	6	95	80	5	-15	-15	1.92		73.1	0.216	IRRLA
4326-13 4327-01	Aloha Village TMK 3-9-16-25	D V O	2005	204318	1562646	ROT	6	80	66	4	-14	-14	1.56		72.3	0.18	IRRLA
4327-01	TMK 3-9-17-37	Dang Y O Toba J	1947 1947	204352 204356	1562702 1562707		8 10	38 31									UNU
4327-03	TMK 3-9-05-51	Flood E	1949	204334	1562707		8	26									IRR
4327-04	TMK 3-9-05-25	Brown J	1949	204334	1562703		8	22									IRR
4327-05	TMK 3-9-05-22	Murphy F	1949	204341	1562704		8	38									IRR OTH
4327-06	TMK 3-9-16-03	Machida S	1967	204352	1562705		-	45									IRR
4327-07	Kihei-Akahi	Kihei Akahi Condo Assc.	2000	204344	1562705	ROT	6	80	56	5		-24	1.12		73.4	0.216	IRRPA
4327-08	Kalama Beach A1		2009	204400	1562717	ROT	2	16								0.210	OBS
4327-09	Kalama Beach A2		2009	204400	1562717	ROT	2	45									OBS
4327-10	Kalama Beach A3		2009	204400	1562717	ROT	2	70									OBS
4426-02	Kihei Injection	Maui DPW	1974	204406	1562626	PER	18	230	109	-51		-121	4				OTH
4427-01 4427-04	TMK 3-9-05-52 TMK 3-9-11-38	Maui County		204414	1562722												IRR
4427-04	TMK 3-9-02-02	Nishiji R Akina F	1949 1950	204423	1562725		8	24									IRR
4427-03	TIVIN 3-9-02-02	AKIIIA F	1950	204417	1562701		8	82									UNU
WELLS TO	THE NORTH THAT W	ILL NOT BE IMPACTED E	BY HONU	A'ULA PRO	DJECT WELLS												
4422-01	Waiohuli	USGS	2001	204419	1562205	ROT	4	1940	1864	30	-60	-76	5.58				OBS
4424-01	Keokea Highlands 2	Maui Highlands Prop, LLC		204459	1562502	ROT	8	577	553	-4	-24	-24	2.6			0.432	MUNPR
4425-01	Keokea Highlands	Maui Highlands LLC	2004	204459	1562502	ROT	6	570	551	1	-19	-19	2.76			0.432	MUNPR
4426-01	Kihei Inject TH	Maui Dpw	1972	204420	1562641	ROT	2	203									OBS
4426-03	Kihei-Maui R&T	Maui R&T Part	1990	204456	1562641	ROT	8	157	124	-3	-33	-33	1.87	369	20		IRRLA
4427-02	TMK 3-9-02-8	Akina F	1945	204429	1562707		8	30									UNU
4427-03	Medo	Miranda H	1948	204438	1562718		10	22									UNU
4427-06	Kihei Fire B1	2009	204416	1562722	ROT		2	16									OBS
4427-07	Kihei Fire B2 Kihei Fire B3	2009	204416	1562722	ROT		2	40									OBS
4427-08	Alliel Fire 63	2009	204416	1562722	ROT		2	70									OBS
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Bottom of Hole Static Head Case Total Ground Bottom Bottom Installed Chloride Temp Year Drilled Depth ft. Dia. Elev Solid Perf Capacity Well No. Well Name Owner/User Type Latitude Longitude in. ft. Casing Casing ft MSL ft MSL (MGD) Use Kihei Baptist Chapel TMK 3-9-02-36 Tmk 3-9-02-32 Tmk 3-9-01-02 1978 1945 1946 1947 204433 204529 204519 204533 1562721 1562716 1562714 1562740 Kihei Baptist Chapel 20 6 8 IRRLA AGRCP IRR 4427-09 15 30 35 20 47 70 28 42 71 14 20 86 45 11 50 1832 1815 0.021 ei Baptist Cha Akina R Yee W Perreira L Maui County Maui County 4527-01 4527-02 4527-03 4527-04 0.17 UNU UNU UNU IRR UNU Tmk 3-9-08 1948 204504 1562737 1562737 1562738 1562721 1562717 1562711 1562737 1562708 4527-05 Tmk 3-9-08 1948 1948 1949 1990 1992 1989 2001 204503 Tmk 3-9-08
Tmk 3-9-01-9
Tmk 3-9-23-30
Kihei-Pillani
Kihei-Koa
Waiohuli 1
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Table 2

Summary of the Status of Wells Pumping From the Kamaole Aquifer's Basal Lens

	All Wells in the Kamaole Aquifer	134	43	47	44	4.7
akai Corridors	Wells to the North That Will Not Be Impacted by the Honua'ula Project's Wells	45	10	9	6	0.4
Tabulation of Wells in Mauka-to-Makai Corridors	Wells That May Be Impacted by the Honua'ula Project's Wells	55	16	50	9	9.
Tabulation of	Wells to the South That Will Not Be Impacted by the Honua'ula Project's Wells	34	17	5	φ	2.4
	Categories of Well Status	Total Number of Wells	Wells Known or Presumed to be in Use	Wells Known to No Longer be in Use or Do Not Draw From the Basal Lens	Wells of Unknown Status Relative to Their Use	Estimated Average Pumpage in MGD

Notes: 1. Locations of all the wells are shown on Exhibit A and Exhibit B is a complete listing of all wells.

- Wells known to no longer be in use or do not draw from the basal lens include: those listed as unused, abandoned, sealed, or lost in Exhibit B; high elevation tunnels which tap perched groundwater; wells used for observation only; and production wells not yet in service. This latter group includes the Honua'ula project's four completed wells.
- Pumpage amounts were estimated from reported pumpage (Figures 1, 2, and 3) and assigning an
 average of 0.075 MGD for condominium and hotel irrigation wells and lesser amounts to wells
 serving smaller parcels.



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MEMORANDUM

Tom Schnell - PBR Charlie Jencks - Honua'ula Partners

To:

Tom Nance

From:

Subject: Cost of the Potable and Non-Potable Systems

This memo provides estimates of the cost to construct and operate the Honua'ula project's potable and non-potable systems in order to address Comment 17 of the Planning Commission. You can use Figure 4 in my December 2009 Water Systems master plan report in creating the map requested in Comment 12.

A number of assumptions had to be made to create cost estimates for the project's consumers at this stage of the project's planning. The most significant of these are:

- The analysis is based on full build-out of the project.
- The estimated potable and non-potable infrastructure cost is \$21 million (refer to Table 1). Not included in this estimate are: construction and installed pumps in the two onsite wells (these were done in the early 1990s); and all onsite potable and non-potable pipelines throughout the project.
- The \$21 million infrastructure cost is recovered in water sales at 6 percent over 20 years
- Groundwater pumpage and RO treatment are 10 percent greater than actual water sales to account for leakage and unmetered use.
- Power for well pumps, booster pumps, and the RO plant is purchased from MECO at an average of \$0.30/KWH.
- Pump efficiencies are 78 percent and the motors driving them are 87 percent efficient.

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Sixty-seven (67) percent of the feedwater through the RO plant is recovered as drinking water and 33 percent is concentrate which is reused for golf course irrigation.

- Costs for operating personnel, administration, and miscellaneous supply and maintenance will be 40 percent of the power to operate the pumps and RO plant.
- Purchase of non-potable (brackish) and RO-treated potable water by customers throughout the
 project will be distributed as follows:

Projected Water Sales in Average MGD

Water	Service Pre	Service Pressure Zone	Both
System	640' Zone	810' Zone	Zones
Potable	0.260	0.082	0.342
Non-Potable	0.556	0.254	0.810
Total for Both	0.816	0.336	1.152

Based on the foregoing set of assumptions, the daily operating cost for both systems in both service zones would be \$3,000. The daily cost of capital recovery would be \$4,950. Generally where dual water service is available, the sale price for potable water is about double the price for non-potable water. Using that as a guideline, the cost to consumers, with and without capital recovery and ignoring a profit to the utility, would be as follows:

Estimated Cost in Dollars Per Thousand Gallons

	Dotable	Non Dotable
Cost Items Included	Water	Water
Based on Operation and Maintenance Exclusively (No Capital Recover)	\$ 4.00	\$ 2.00
Based on Operation, Maintenance, and Full Capital Recovery	\$ 10.64	\$ 5.32

Attachment

Table 1

Estimated Cost of the Honua'ula Water System Infrastructure

Infrastructure Item	Amount
Construction and Testing of Kamaole Wells 1 to 4	\$ 1,570,000
Outfit Kamaole Wells 1 to 4	3,000,000
Offsite Pipeline and Access Road, Wells to 640' Tank	6,500,000
RO Plant and Potable and Non-Potable Tanks at 640'	4,200,000
Potable and Non-Potable Booster Pumps at 640' Tank	200'000
Pipelines and Access Road, 640' to 810' Tanks	750,000
Potable and Non-Potable Tanks at 810'	1,850,000
Total for Construction	\$ 18,370,000
Engineering and Construction Management	2,630,000
Total	\$ 21,000.000